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European Technical Assessment

ETA-22/0573 of 22/12/2022

General Part:

| Technical Assessment Body issuing the European Technical Assessment: | SINTEF |
|---|---|
| Trade name of the construction product: | HRC400 Series Rebar Couplers |
| Product family to which the construction product belongs: | Couplers for mechanical splices of reinforcing steel bars |
| Manufacturer: | HRC Europe Lierstranda 107 NO-3412 Lierstranda Norway |
| Manufacturing plants: | HRC Europe AS Lierstranda 107 N-3412 Lierstranda Norway |
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| This European Technical Assessment contains: | 8 pages including 3 annexes which form an integral part of this assessment |
| This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of: | EAD 160129-00-0301, Edition 01/2020 |

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1. TECHNICAL DESCRIPTION OF THE PRODUCT

The HRC400 Rebar Couplers are used as a mechanical splice for connecting reinforcing bars in reinforced concrete structures under static or quasi-static, fatigue and low cycle loading.

The product description is given in Annex A.

The characteristic material values, dimensions and tolerances of HRC400 Rebar Couplers not indicated in Annex A shall correspond to the respective values laid down in the technical documentation^[1] of this European Technical Assessment.

[1] The technical documentation of this European technical assessment is deposited at SINTEF and, as far as relevant for the tasks of the approved bodies involved in the attestation of conformity procedure, is handed over to the approved bodies.

2. SPECIFICATION OF THE INTENDED USE(S) IN ACCORDANCE WITH THE APPLICABLE EUROPEAN ASSESSMENT DOCUMENT (EAD)

The performances given in Section 3 are only valid if the HRC400 Rebar Couplers are used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the HRC400 Rebar Couplers of at least 100 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3. PERFORMANCE OF THE PRODUCT AND REFERENCES TO THE METHODS USED FOR ITS ASSESSMENT

| No | Essential characteristic | Performance |
|----|--|-------------------------|
| 1 | Resistance to static or quasi-static loading | See Annex C |
| 2 | Slip under static or quasi-static load | See Annex C |
| 3 | Slip after static or quasi-static load | See Annex C |
| 4 | Fatigue strength for N = $2 \cdot 10^6$ load cycles | No performance assessed |
| 5 | Fatigue strength for S-N curve with k_1 and k_2 according to EN 1992-1-1 | No performance assessed |
| 6 | Fatigue strength for S-N curve with specific k_1 and k_2 | See Annex C |
| 7 | Resistance to low cycle loading (seismic actions) | See Annex C |

3.1 Mechanical resistance and stability (BWR 1)

3.2 Safety in case of fire (BWR 2)

| No | Essential characteristic | Performance |
|----|--------------------------|-------------|
| 8 | Reaction to fire | Class A1 |

4. ASSESSMENT AND VERIFICATION OF CONSTANCY OF PERFORMANCE (AVCP) SYSTEM APPLIED, WITH REFERENCE TO ITS LEGAL BASE

In accordance with European Assessment Document EAD 160129-00-0301, the applicable European legal act is: 2000/206/EC.

The system to be applied is: 1+

5. TECHNICAL DETAILS NECESSARY FOR THE IMPLEMENTATION OF THE AVCP SYSTEM, AS PROVIDED FOR IN THE APPLICABLE EAD

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at SINTEF.

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Ву

SINTEF

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The following standards are referred to in this ETA:

- EN 1992-1-1:2004+AC:2010+A1:2014

Eurocode 2: Design of concrete structures - Part 1-1: General rules and rules for buildings

- EN 1998-1:2004+AC:2009+A1:2013

Eurocode 8: Design of structures for earthquake resistance – Part 1: General rules, seismic actions and rules for buildings

- NS-EN 10080:2005 Steel for the reinforcement of concrete. Weldable reinforcing steel. General

- EN 10025-2:2004 + EN 10025-2:2019

Hot rolled products for structural steels – Part 2: Technical delivery conditions for non-alloy structural steels

- EN 10025-3:2019 Hot rolled products for structural steels - Part 3: Technical delivery conditions for normalized rolled weldable fine grain structural steels

ANNEX A – PRODUCT DESCRIPTION

A.1 Design variants

| Nominal rebar diameter | Standard Couplers, HRC410/420 (see Fig. A1) | | | | Positional Couplers, HRC410/490 (see Fig. A2) | | | | | |
|------------------------------|--|------|------|------------------|--|------|------|------------------|----------------------|--|
| Ø | А | В | С | G _{max} | А | D | Е | F _{max} | Torque ¹⁾ | |
| [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [Nm] | |
| 12 | 22 | 22 | 45 | 72 | - | - | - | - | - | |
| 16 | 28 | 28 | 50 | 76 | - | - | - | - | - | |
| 20 | 35 | 35 | 55 | 85 | - | - | - | - | - | |
| 25 | 35 | 42 | 76 | 115 | 35 | 45 | 140 | 280 | 200 | |
| 32 | 45 | 45 | 90 | 130 | 45 | 55 | 157 | 330 | 270 | |
| 40 | 55 | 70 | 105 | 155 | 55 | 70 | 200 | 440 | 330 | |

Table A1: HRC400 Series Rebar Couplers - Design variants

¹⁾ Torque applies only to positional coupler HRC410/490



Figure A1: Standard Coupler, HRC410/420

Figure A2: Positional Coupler, HRC410/490

A.2 Material

Table A2: Material

| Component | Material |
|---------------|---|
| Rebar steel | According to <i>EN 1992-1-1, Annex C</i> . - Reinforcing bars of class B500B or B500C (f _{yk} = 500 MPa) |
| Coupler parts | According to <i>EN 10025-2</i> and <i>EN 10025-3</i> . - Material no.: 1.0538; 1.0590; 1.8901; 1.8903 - Material: S460J0; S450J0; S460N; S460NL |

ANNEX B – INTENDED USE

B.1 Application conditions

HRC400 Series Rebar Couplers are intended for us as mechanical splices of rebar of class B and C according to *EN 1992-1-1* and *Annex C*.

HRC400 Rebar Couplers can be used in design according to the same design assumptions as traditional reinforcing steel (B500B or B500C) without rebar couplers.

- Transfer of static or quasi-static tensile and compression loads according to EN 1992-1-1, clause 8.7 and 8.8(4)
- Slip limitation for control of crack width according to *EN 1992-1-1, clause 7.3*
- Resistance to high-cycle fatigue loading according to EN 1992-1-1, clause 6.8.4
- Resistance to low-cycle seismic loading according to EN 1998-1, clause 5.6.3(2)

B.2 Installation criteria

- The same concrete cover as for unspliced reinforcement steel bars applies also to the couplers.
- Rebar bends shall start in a distance of at least 2 x nominal rebar diameter away from the connection of the coupler part to the rebar (weld collar).
- HRC400 couplers shall be applied as delivered, without any modification or replacement of parts.
- HRC400 coupler parts are to be stored with thread protection. Couplers fixed in the reinforcing work shall have the thread protection in place until the splice is assembled.
- Installation steps have to be followed.

Standard Coupler, HRC 410/420, see Fig. A1

- 1. Remove plastic-plug/cap
- 2. Check for damage/remove dirt etc.
- 3. Screw couplers firmly together by hand (possible to tighten, e.g. by pipe wrench)
- 4. Visual control: \rightarrow no full thread visible

Positional Coupler, HRC 410/490, see Fig. A2

- 1. Make sure sleeve is positioned as far back onto the straight threaded bolt as possible.
- 2. Remove plastic-plug/cap
- 3. Check for damage/remove dirt etc.
- 4. Position sleeve at male coupler end
- 5. Turn sleeve onto taper threaded end by hand
- 6. Make sure all threads are engaged on tapered end
- 7. Visual control: \rightarrow no full thread visible on tapered end
- 8. Apply torque, see table B1
- 9. Turn lock nut firm onto sleeve, apply torque, see table B1

Table B1: Positional Coupler – recommended torque during installation

| Nominal rebar diameter, Ø | [mm] | 25 | 32 | 40 | |
|---------------------------|------|-----|-----|-----|--|
| Recommended torque | [Nm] | 200 | 270 | 330 | |

ANNEX C – ESSENTIAL CHARACTERISTICS

| | ameter | Resistance quasi stat | to static or ic loading | Slip | | Fatigue strength ⁵⁾ | | | Resistance to low cycle loading (seismic action) | | | |
|---------------------------|-------------|--------------------------|----------------------------|------------------|------------------|---|-------------------------|-----|---|---|-------|----------------------|
| | l Rebar dia | Failure of rebar | Failure of coupler | under Ioading | after Ioading | (S-N-curve with specific defined k1 and k2) | | | Ultimato Ioad, I | e tensile _{u,min} ⁷⁾ | | |
| upler type | Nominal | 1),bar,outside | ,,coupler ²) | S1 ³⁾ | S2 ⁴⁾ | | | | | | | : mode ⁸⁾ |
| Cop | Ø | f _{u,min} | f _{u,min} | | | $\Delta \sigma_{ m Rsk}$ | k1 | k2 | u ₂₀ ⁶⁾ | B500B | B500C | ailure |
| | [mm] | [MPa] | [MPa] | [mm] | [mm] | [MPa] | | | [mm] | [kN] | [kN] | ű |
| | 12 | | > 850 | | < 0,10 | | 19 10 ⁷) | | | 61,1 | 65,0 | rebar outside splice |
| pler 20 | 16 | B: 540 C: 575 | > 850 | < 0,10 | | 49 | | | | 108,6 | 115,6 | |
| l Cou 10/42 | 20 | | > 850 | | | $(N = 10^{\circ})$ 69 $(N = 2 \cdot 10^{\circ})$ 4, | 16 | | 0.2 | 169,6 | 180,6 | |
| idard RC 41 | 25 | 35001 | > 850 | | | | 4,0 | 0,5 | 0,2 | 265,1 | 282,3 | |
| Star H | 32 | | > 740 | > 740 | | (, | | | | 434,3 | 462,4 | |
| | 40 | | > 850 | | | | | | | 678,6 | 722,6 | of the |
| onal Coupler C 410/490 | 25 | 40 75 | | | | 49 | | | | 265,1 | 282,3 | ipture o |
| | 32 | 500B: 5 | > 670 | < 0,10 | < 0,10 | (N = 10') 69 | 4,6 | 8,3 | 0,2 | 434,3 | 462,4 | uctile ru |
| Positi HR | 40 | 9. 19 | | | | $(N = 2 \cdot 10^6)$ | | | | 678,6 | 722,6 | D |

¹⁾ f_{u,min,bar,outside} according to EN 1992-1-1, Annex C.1:

For B500B: $f_{u,min,bar,outside}$ = $k_{B500B} \cdot f_{yk}$ = 1,08 \cdot 500 MPa = 540 MPa

For B500C: $f_{u,min,bar,outside} = k_{B500C} \cdot f_{yk} = 1,15 \cdot 500 \text{ MPa} = 575 \text{ MPa}$

Failure loads are determined by the strength of the parent rebar, not the HRC400 mechanical coupler. The full specified elongation A_{gt} of the rebar can be developed, according to *EN 1992-1-1*, *Annex C.1*.

- ²⁾ f_{u,min,coupler} = minimum rebar stress equivalent to failure of the coupler. Values from test results with larger rebar than the coupler are intended for ("oversized rebar"). The full actual elongation A_{gt,act} of the intended rebar size will be developed.
- ³⁾ Slip across the mechanical splice under loading at 0,6 \cdot f_{yk} = 0,6 \cdot 500 MPa = 300 MPa
- ⁴⁾ Slip across the mechanical splice after unloading from $0.6 \cdot f_{yk}$ to a load level of $0.02 \cdot f_{yk} = 0.02 \cdot 500$ MPa = 10 MPa
- $^{5)}$ $\,$ Fatigue strength $\Delta\sigma_{Rsk}$ for S-N-curve with specific defined stress exponents k_1 and k_2
- ⁶⁾ u_{20} = Residual max deformation
- ⁷⁾ $F_{u,min} = A_{s,nom,bar,outside} \cdot f_{u,min,bar,outside} = \pi/4 \cdot 0/2 \cdot f_{u,min,bar,outside}$
- ⁸⁾ The actual failure loads are determined by the strength of the parent rebar, not the HRC400 couplers. Splices of rebar with lower/higher actual tensile strength will therefore achieve lower/higher actual capacities than given in the table. The failure mode remains unchanged: ductile rupture of the parent rebar.